Reports of Enlarged Session of the Seminar of I. Vekua Institute of Applied Mathematics Volume 23, 2009

## THE STRESSES CONCENTRATION PROBLEM FOR CYLINDRICAL SHELLS ON THE I. VEKUA'S HIGH APPROXIMATIONS

Gogia A., Meunargia T.

**Abstract**. In the present paper on the basis of I. Vekua's theory (approximate N = 0, 1, 2) we consider well-known problem of stresses concentration for shallow and non-shallow cylindrical shell. To solve the problems algorithm of full automation is devised by means of the net method. The program named VEKMUS is constructed [2]. By means of the program the problems of stresses concentration shallow and non-shallow cylindrical shells are solved for the approximations N = 0, 1, 2.

Keywords and phrases: Shallow and non-shallow shells, stress concentration.

## AMS subject classification (2000): 74K25.

- I. Vekua's complete system of 2-D equations of non-shallow cylindrical shells for the any approximation of order N have the form [1]:
  - a) equations of equilibrium

$$\begin{cases}
\nabla_{\alpha} \overset{(m)}{\sigma_{1}^{\alpha}} - \frac{2m+1}{h} \binom{m-1}{\sigma_{1}^{3}} + \overset{(m-3)}{\sigma_{1}^{3}} + \cdots + F_{1} = 0, \\
\nabla_{\alpha} \overset{(m)}{\sigma_{2}^{\alpha}} + \frac{1}{R} \overset{(m)}{\sigma_{2}^{3}} - \frac{2m+1}{h} \binom{m-1}{\sigma_{2}^{3}} + \overset{(m-3)}{\sigma_{2}^{3}} + \cdots + F_{2} = 0, \\
\nabla_{\alpha} \overset{(m)}{\sigma_{3}^{\alpha}} - \frac{1}{R} \overset{(m)}{\sigma_{2}^{3}} - \frac{2m+1}{h} \binom{m-1}{\sigma_{3}^{3}} + \overset{(m-3)}{\sigma_{3}^{3}} + \cdots + F_{3} = 0,
\end{cases} \tag{1}$$

where

$$\begin{pmatrix} \binom{m}{\sigma_{j}^{i}}, \binom{m}{\phi_{j}} \end{pmatrix} = \frac{2m+1}{2h} \int_{-h}^{h} \left( \sqrt{\frac{g}{a}} \, \sigma_{j}^{i}, \sqrt{\frac{g}{a}} \, \phi_{j} \right) P_{m} \left( \frac{x_{3}}{h} \right) dx_{3}, \quad (i, j = 1, 2, 3),$$

$$\begin{pmatrix} \binom{m}{F_{i}} = \binom{m}{\phi_{i}} + \frac{2m+1}{2h} \left[ \sqrt{\frac{g_{+}}{a}} \, \sigma_{i}^{3} - (-1)^{m} \sqrt{\frac{g_{-}}{a}} \, \sigma_{i}^{3} \, \right], \quad \sqrt{\frac{g_{\pm}}{a}} = 1 \pm \frac{h}{R},$$

$$\begin{pmatrix} \binom{\pm}{a} = \frac{1}{2h} & \binom{\pm}{a} \\ \sigma^{3} = \sigma^{3} (x^{1}, x^{2}, \pm h), \quad (m = 0, 1, ..., N),$$

$$-h < x_{3} = x^{3} < h.$$

b) For the Hooke's law we have [3]

$$\sigma_{11}^{(m)} = (\lambda + 2\mu) \left[ \partial_1^{(m)} u_1 + \frac{h}{R} \partial_1^{(m)} \underline{u_1} \right] + \lambda \left[ \left( \partial_2^{(m)} u_2 + \frac{1}{R} \underline{u_3}^{(m)} \right) + \underline{u_3'} + \frac{h}{R} \underline{u_3''} \right],$$

$$\sigma_{12}^{(m)} = \mu \left[ \partial_2^{(m)} u_1 + \partial_1^{(m)} u_2 + \frac{h}{R} \partial_1 \underline{u_2}^{(m)} \right],$$

$$\sigma_{21}^{(m)} = \mu \left[ \partial_1^{(m)} u_2 + \sum_{s=0}^{N} A_{ms} \partial_2^{(s)} \underline{u_1} \right],$$

$$\sigma_{22}^{(m)} = (\lambda + 2\mu) \sum_{s=0}^{N} A_{ms} \left( \partial_{2} u_{2}^{(s)} + \frac{1}{R} u_{3}^{(s)} \right) + \lambda \left( \partial_{1} u_{1}^{(m)} + u_{3}^{(m)} \right),$$

$$\sigma_{13}^{(m)} = \sigma_{31}^{(m)} = \mu \left[ \partial_{1}^{(m)} u_{3}^{(m)} + u_{1}^{(m)} + \frac{h}{R} \partial_{1} \underline{u_{2}}^{(m)} \right], \quad \sigma_{23}^{(m)} = \mu \left[ u_{2}^{(m)} + \sum_{s=0}^{N} A_{ms} \left( \partial_{2} u_{3}^{(m)} - \frac{1}{R} u_{3}^{(s)} \right) \right],$$

$$\sigma_{32}^{(m)} = \mu \left[ u_{2}^{(m)} + \frac{h}{R} u_{2}^{(m)} + \partial_{2} u_{3}^{(m)} - \frac{1}{R} u_{2}^{(m)} \right],$$

$$\sigma_{33}^{(m)} = \mu \left[ \partial_{1}^{(m)} u_{1}^{(m)} + \partial_{2}^{(m)} u_{2}^{(m)} + \frac{1}{R} u_{3}^{(m)} + \frac{h}{R} \partial_{1} \underline{u_{1}}^{(m)} \right] + (\lambda + 2\mu) \left( u_{3}^{(m)} + \frac{h}{R} u_{3}^{(m)} \right),$$
(2)

where

Here  $Q_s(x)$  is the Legendre function of second order.

For shallow cylindrical shells  $\partial_{\alpha} \underline{u^i}^{(m)} = 0$  and  $u_i^{(m)} = 0$ .

On bases I. Vekua's approximate N=0,1,2 an automatic numerical program named VEKMUS is compiled to calculate the stress concentration for the cylindrical shells weakened by rectangular holes.

Let  $u_i = u_i(x_1, x_2)$  are two-differentiable functions on some domain  $\omega$ .  $A_{klm} = A_{klm}(x_1, x_2)$ ,  $f_k = f_k(x_1, x_2)$  are given continuous functions on the same domain  $\omega$ ,  $k, l = \overline{1, n}, m = \overline{1, 6}$ . In the program VEKMUS the algorithm of interchanging the following system equations by the finite-difference scheme is constructed:

$$A_{k11} \frac{\partial^{2} u_{1}}{\partial x_{1}^{2}} + A_{k12} \frac{\partial^{2} u_{1}}{\partial x_{1} \partial x_{2}} + A_{k13} \frac{\partial^{2} u_{1}}{\partial x_{2}^{2}} + A_{k14} \frac{\partial u_{1}}{\partial x_{1}} + A_{k15} \frac{\partial u_{1}}{\partial x_{2}} + A_{k16} u_{1}$$

$$+ A_{k21} \frac{\partial^{2} u_{2}}{\partial x_{1}^{2}} + A_{k22} \frac{\partial^{2} u_{2}}{\partial x_{1} \partial x_{2}} + A_{k23} \frac{\partial^{2} u_{2}}{\partial x_{2}^{2}} + A_{k24} \frac{\partial u_{2}}{\partial x_{1}} + A_{k25} \frac{\partial u_{2}}{\partial x_{2}} + A_{k26} u_{2}$$

$$+ A_{kn1} \frac{\partial^{2} u_{n}}{\partial x_{1}^{2}} + A_{kn2} \frac{\partial^{2} u_{n}}{\partial x_{1} \partial x_{2}} + A_{kn3} \frac{\partial^{2} u_{n}}{\partial x_{2}^{2}} + A_{kn4} \frac{\partial u_{n}}{\partial x_{1}} + A_{kn5} \frac{\partial u_{n}}{\partial x_{2}} + A_{kn6} u_{n} = f_{k}.$$
(3)

It is easy to see that of Vekua's shell theory or plane elasticity theory are particular cases of the system (3). By this reason above mentioned algorithm will be served

equally by this two theories. The corresponding to (3) finite-difference scheme will be constructed automatically in the matrix-vector form

d automatically in the matrix-vector form 
$$\begin{cases} B_0W_0 + C_0W_1 = F_1, \\ A_1W_0 + B_1W_1 + C_1W_2 = F_2, \\ A_2W_1 + B_2W_2 + C_2W_3 = F_3, \\ \dots & \\ A_{M-1}W_{M-2} + B_{M-1}W_{M-1} + C_{M-1}W_M = F_{M-1}, \\ A_MW_{M-1} + B_MW_M = F_M, \end{cases}$$

$$(4)$$

$$B_i, C_i, i = \overline{0, M}, \text{ are the quadratic matrix of order nN of the same structure,}$$

where  $A_i$ ,  $B_i$ ,  $C_i$ ,  $i = \overline{0, M}$ , are the quadratic matrix of order nN of the same structure,

$$F_i = [(f_1, f_2, \dots, f_n)_{i0}, (f_1, f_2, \dots, f_n)_{i1}, \dots, (f_1, f_2, \dots, f_n)_{iN}],$$

$$W_i = [(u_1, u_2, \dots, u_n)_{i0}, (u_1, u_2, \dots, u_n)_{i1}, \dots, (u_1, u_2, \dots, u_n)_{iN}],$$

are the vectors of order nN, n is the number of unknowns on nodal points of the net, N and M are the discretization parameters of the net.

From the system (4) we get easily the algorithm of its solution

$$W_0 = B_0^{-1}(F_0 - C_0 W_1), \quad W_1 = X_1 W_2 + Y_1, W_i = X_i W_{i+1} + Y_i, \qquad W_M = X_M Y_M, \quad (i = 2, 3, ..., M-1),$$
(5)

where

$$X_{1} = -(B_{1} - A_{1} B_{0}^{-1} C_{0})^{-1} C_{1}, \quad Y_{1} = (B_{1} - A_{1} B_{0}^{-1} C_{0})^{-1} (F_{1} - A_{1} B_{0}^{-1} F_{0}),$$

$$X_{i} = -(A_{i} X_{i-1} + B_{i})^{-1} C_{i}, \quad Y_{i} = (A_{i} X_{i-1} + B_{i})^{-1} (F_{i} - A_{i} Y_{i-1}),$$

$$X_{M} = -(A_{M} X_{M-1} + B_{M})^{-1}, \quad Y_{M} = F_{M} - A_{M} Y_{M-1}, \quad (i = 2, 3, ..., M-1).$$
(6)

The formulas (5) and (6) are algorithms of matrix factorization. On the first step the coefficients  $X_1, Y_1, \ldots, X_M, Y_M$  are calculated (the direct step), and on the second step the desired vectors  $W_M, W_{M-1}, \dots, W_0$  are calculated (inverse step).

The stresses concentration problem. Let we have the cylindrical shell (Fig. 1), the lateral surfaces of which are loading by uniformly distributed tension force Q. Denote by P the intensity of this force. Other surfaces are free. Determine the stress state of the shell.

boundary curves	boundary conditions	
AB,CD	$\sigma_{11}^{(0)} = p,  \sigma_{ij}^{(m)} = 0,  m = 0, 1, 2$	
$AD, BC, A_1D_1, B_1C_1, A_1B_1, D_1C_1$	$\sigma_{ij}^{(m)} = 0, \ m = 0, 1, 2$	

The formulated problem was solved by VEKMUS for shallow and non-shallow shells on the domain  $\omega_h^{100}(h=\frac{1}{100})$ .

From the obtained results we give the short analysis of the solutions.

The concentration of stress are high valued at the neighbourhoods of the points  $A_1$ ,  $B_1, C_1, D_1.$ 

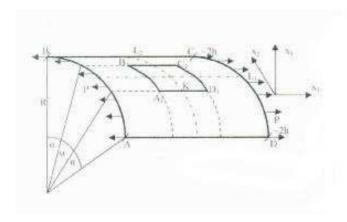


Fig.1.

	$ max \sigma_{11} $	$max \sigma_{12} $	$ max \sigma_{22} $	$max \sigma_{11}(k) $
N=0 shellow shells	21p	17p	18p	1,5p
N=0 nonshellow shells	21 <i>p</i>	17p	18p	1,5p
N=1 shellow shells	70p	63p	74 <i>p</i>	4p
N=1 nonshellow shells	69p	62p	73 <i>p</i>	4p
N=2 shellow shells	29p	27p	33p	3,9p
N=2 nonshellow shells	37p	34p	40p	2,5p

$$E = 2, 1 \cdot 10^6$$
;  $\sigma = 0.3$ ;  $\alpha = \pi/6$ ;  $R = 200$  sm;  $2h = 4$  sm;  $|AB| = |AD| = 200^* \pi/2$  sm,  $|A_1B_1| = |A_1D_1| = 200^* \pi/6sm, P = 1$ .

## REFERENCES

- 1. Vekua I. Shell theory: General Methods of Construction. *Pitman advanced publishing program*, *Boston–London–Melbourne–New York*, 1985.
- 2. Gogia A. Computation of a cylindrical shell with holes. Bull. Georgian acad. sci.  $\mathbf{115}$  (1984), 41-44
- 3. Gogia A., Meunargia T. Some problem of the stresses concentration for non-shallow cylindrical shells on the basis of I. Vekua's theory. AMIM, 1 (2007), 39-51.

Received 14.05.2009; revised 19.10.2009; accepted 21.11.2009.

Authors' address:

A. Gogia and T. Meunargia

I. Vekua Institute of Applied Mathematics of

Iv. Javakhishvili Tbilisi State University

2, University St., Tbilisi 0186

 ${\bf Georgia}$ 

E-mail: tengiz.meunargia@viam.sci.tsu.ge